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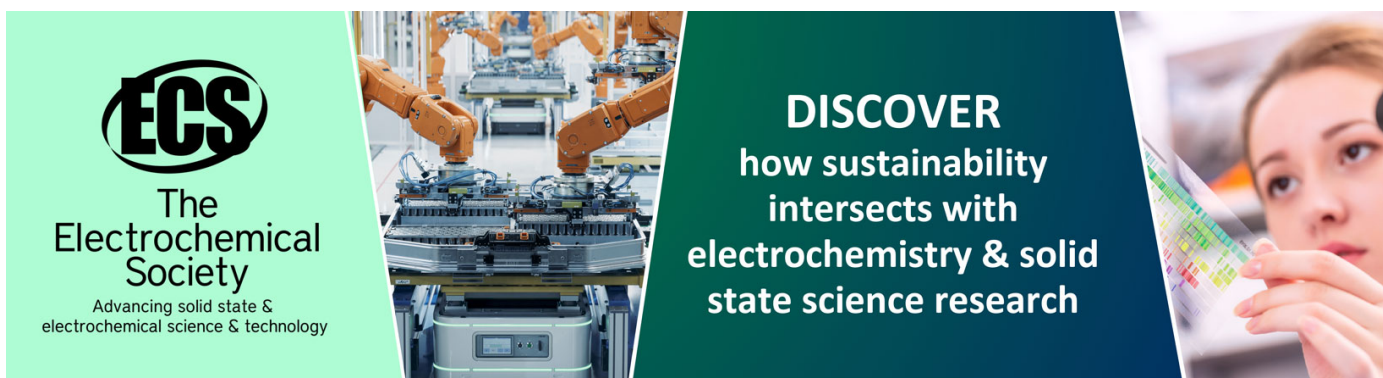
Investigation of the nonlinear problem of deforming sandwich plate with transversally soft core

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Investigation of the nonlinear problem of deforming sandwich plate with transversally soft core

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Abstract. The study of the problem of finding the stress-strain state of a sandwich plate with a transversally soft core under the action of a transverse load in a one-dimensional geometrically nonlinear formulation was carried out. A generalized formulation of the problem in the form of an operator equation is proposed. The properties of the operator of the equation are established, which make it possible to use the general results of the theory of monotone operators in the study of correctness. To find the stress-strain state of the plate, a two-layer iterative method is proposed with lowering the nonlinearity on the lower layer. A finite-dimensional approximation of the problem and the iterative method was carried out. The convergence of finite-dimensional approximations and the iterative method has been studied. For the numerical implementation of the proposed approximate methods, a software package has been developed in the Matlab environment. Based on it, numerical experiments were carried out.

1. Introduction

Multilayer structures, in particular plates, are widely used in various areas of modern technology: aerospace, aviation, shipbuilding; industrial, civil and transport construction, chemical and power engineering [1–6]. Interest in laminated plates is primarily due to the fact that they have a set of properties and features that qualitatively distinguish them from traditional structures. Multilayer structures usually consist of different materials with significantly different physical and mechanical properties. For bearing layers, as a rule, materials with high elastic moduli are used, which perceive the main part of external force effects. The core serves for the monolithic structure and provides the redistribution of forces between the carrier layers, as well as performs a number of other functions, for example, protection from radiation, heat and sound protection, etc. [7–10]

In this paper, we consider a sandwich plate consisting of two external carrier layers and a transversally soft core located between them and connected to the carrier layers by means of adhesive bonding. A generalized formulation of the problem in the form of an operator equation is proposed. The properties of the operator of the equation are established. This made it possible, in the study of correctness, to use the general results of the theory of monotone operators [11]. To find the stress-strain state of the plate, a two-layer iterative method is proposed with lowering the nonlinearity on the lower layer. A finite-dimensional approximation of the problem and the iterative method was carried out. The convergence of finite-dimensional approximations and the iterative method has been studied.



For the numerical implementation of the proposed approximate methods, a software package has been developed in the Matlab environment. Based on it, numerical experiments were carried out. Note that a physically nonlinear problem of determining the equilibrium position of a soft network shells and methods of solving them have been studied in [12–15]. Geometrically nonlinear problems were considered in [16–20]. The nonlinear problems of the shells theory were studied in [21–26]. The case when both ends of the plate are rigidly fixed was studied in [12, 27].

2. Problem statement

We consider the problem of determining the stress-strain state of an infinitely wide sandwich plate with a transversally soft core. The plate length is equal a , the thickness of the aggregate is $2h$, the thickness of the supporting layers are equal $2h_{(k)}$, where k is the layer number. The study of the processes of deformation of such elements, first of all, is dictated by the need to determine the degree of their suitability for further use. To describe the stress-strain state in bearing layers, the equations of the Kirchhoff-Love model are used; in the filler, the equations of elasticity theory, simplified within the accepted model of the transversally soft layer and integrated across the thickness with satisfaction of the conjugation conditions of the layers by displacement. In accordance with [8, 9], we introduce the following notation: $H_{(k)} = h + h_{(k)}$ (Hereinafter we assume that $k = 1, 2$), $X_{(k)}^1$, $X_{(k)}^3$ are the components of the surface load, reduced to the middle surface of the k -th layer, $w^{(k)}$, $u^{(k)}$ are the deflections and axial displacements of points the middle surface of the k -th layer, respectively, $T_{(k)}^{11}$, $M_{(k)}^{11}$ are membrane forces and internal bending moments in the k -th layer, respectively. The edges of the plate are assumed to be fixed, so that the conditions $u^{(k)}(x) = 0$, $w^{(k)}(x) = d w^{(k)} / dx = 0$ for $x = 0$, $x = a$ are satisfied. We consider the geometrically nonlinear case: $M_{(k)}^{11} = D_{(k)} d^2 u^{(k)} / dx^2$, $T_{(k)}^{11} = B_{(k)} (d u^{(k)} / dx + 0.5 (d w^{(k)} / dx)^2)$, where $B_{(k)} = 2h_{(k)} E^{(k)} / (1 - \nu_{12}^{(k)} \nu_{21}^{(k)})$ is the tension-compression stiffness of the k -th layer, $E^{(k)}$ and $\nu_{12}^{(k)}$, $\nu_{21}^{(k)}$ are the first-kind modulus of elasticity and the Poisson coefficients of the material of the k -th carrier layer, $D_{(k)} = B_{(k)} h_{(k)}^2 / 3$ is the flexural rigidity of the k -th layer. Let $U = (w^{(1)}, w^{(2)}, u^{(1)}, u^{(2)})$ be the vector of displacements of the points of the middle surface of the k -th layer, be q^1 the tangential stresses in the core. For q^1 we assume that the boundary conditions $q^1(0) = q^1(a) = 0$ are satisfied. In [28, 29], to describe the stress-strain state of a three-layer plate, the potential energy strain functional was constructed:

$$L(U, q^1) = P(U, q^1) - A(U, q^1) - A_q(U, q^1), \quad P(U, q^1) = \frac{1}{2} \int_0^a \left\{ \sum_{k=1}^2 [B_{(k)} (d u^{(k)} / dx + \frac{1}{2} (d w^{(k)} / dx)^2)]^2 + D_{(k)} (d^2 w^{(k)} / dx^2)^2 + c_1 (q^1)^2 + c_2 (d q^1 / dx)^2 + c (w_3^{(2)} - w_3^{(1)})^2 \right\} dx$$

is the potential energy of deformation, G_{13} , E_3 are the modules of transverse shear and compression of the core, $c_1 = 2h / G_{13}$, $c_2 = h^3 / 3E_3$, $c_3 = E_3 / (2h)$, $A(U, q^1) = \int_0^a \sum_{k=1}^2 [X_{(k)}^1 u^{(k)} + M_{(k)}^1 d w^{(k)} / dx + X_{(k)}^3 w^{(k)}] dx$ is the work of given external forces and moments, $M_{(k)}^1$ is the surface moment of external forces reduced to the middle surface of the k -th layer,

$$A_q(U, q^1) = \int_0^a [(u^{(1)} - u^{(2)}) - \sum_{k=1}^2 H_{(k)} d w^{(k)} / dx + c_1 q^1 - c_2 d^2 q^1 / dx^2] q^1 dx$$

is the work of unknown contact tangential stresses at the corresponding displacements. It was established [29] that the solution of the problem of equilibrium of a sandwich plate are stationary points of the functional L .

3. Generalized statement of the problem

Let $V_k = \overset{o}{W}_2^{(k)}(0, a)$ be the Sobolev spaces [30] with inner products $(u, \eta)_k = \int_0^a d^k u / dx^k d^k \eta / dx^k dx$,

$k = 1, 2$, $V = V_2 \times V_2 \times V_1 \times V_1$. We denote the inner product in V by $(\cdot, \cdot)_V$. The equations for the stationary points of the functional L were obtained by calculating the Gâteaux derivatives [31] of this functional. It was found that the stationary points (U, q^1) are the solution of the variational equation

$$b((U, q^1), (Z, y)) = f(Z) \quad \forall (Z, y) \in W = V \times V_1, \quad (1)$$

where the form $b(\cdot, \cdot)$ given on and the functional f given on V are determined by the formulas

$$\begin{aligned} b((U, q^1), (Z, y)) = & \int_0^a \sum_{k=1}^2 B_{(k)} \left[\frac{du^{(k)}}{dx} + \frac{1}{2} \left(\frac{dw^{(k)}}{dx} \right)^2 \right] \frac{d\eta^{(k)}}{dx} dx + \int_0^a \sum_{k=1}^2 B_{(k)} \left[\frac{du^{(k)}}{dx} + \frac{1}{2} \left(\frac{dw^{(k)}}{dx} \right)^2 \right] \frac{dw^{(k)}}{dx} \times \\ & \times \frac{dz^{(k)}}{dx} dx + \int_0^a \sum_{k=1}^2 D_{(k)} \frac{d^2 w^{(k)}}{dx^2} \frac{d^2 z^{(k)}}{dx^2} dx + \int_0^a \sum_{k=1}^2 c_3 \int_0^a (w^{(2)} - w^{(1)})(z^{(2)} - z^{(1)}) dx + \\ & + \int_0^a \left\{ \sum_{k=1}^2 H_{(k)} \frac{dz^{(k)}}{dx} + (\eta^{(2)} - \eta^{(1)}) \right\} q^1 dx + \int_0^a \left\{ \left[\sum_{k=1}^2 H_{(k)} \frac{dw^{(k)}}{dx} + (u^{(2)} - u^{(1)}) + c_1 q^1 \right] y + \right. \\ & \left. + c_2 dq^1 / dx dy / dx \right\} dx = 0 \quad \forall Z = (z^{(1)}, z^{(2)}, \eta^{(1)}, \eta^{(2)}) \in V, \quad \forall y \in V_1, \end{aligned} \quad (2)$$

$$f(Z) = \int_0^a \sum_{k=1}^2 [X_{(k)}^1 \eta^{(k)} + M_{(k)}^1 \frac{dz^{(k)}}{dx} + X_{(k)}^3 z^{(k)}] dx \quad \forall Z \in V. \quad (3)$$

It is easy to verify that the form $b(\cdot, \cdot)$ is linear in the second argument. In addition, it is limited by the second argument. Therefore, by virtue of the Riesz-Fisher theorem, the form $b(\cdot, \cdot)$ generates an operator $A: W \rightarrow W$ defined by the formula

$$b((U, q^1), (Z, y)) = (A(U, q^1), (Z, y))_W \quad \forall (Z, y) \in W, \quad (4)$$

where $(\cdot, \cdot)_W$ is the inner product in W . Moreover, the following result is true.

Theorem 1. The operator $A: W \rightarrow W$ defined by formulas (2), (4) is bounded.

The functional f defined by formula (3) generates an element $F \in V$ by the formula $(F, Z)_V = f(Z)$, $Z \in V$. Thus, problem (1) can be written as an operator equation

$$A(U, q^1) = (F, 0), \quad (5)$$

4. Investigation of the generalized statement of the problem.

The study of correctness is based on the following results.

The following Sobolev embedding theorem holds [30, p. 68].

Theorem 2. Let $\Omega \subset R^n$ be a bounded domain with a regular boundary Γ , $1 \leq p < \infty$. Then $W_p^{(k)}(\Omega) \subset W_r^{(j)}(\Omega)$ for $0 \leq j < k$ and all r such that $1/p - (k - j)/n \leq 1/r < 1$; besides, for any function $u \in W_p^{(k)}(\Omega)$ the embedding inequality holds

$$\|u\|_{j,r} \leq C_{jr}^{kp} \|u\|_{k,p}, \quad (6)$$

where $\|\cdot\|_{k,p}$ is the norm in $W_p^{(k)}(\Omega)$, and the constant C_{jr}^{kp} depends on Ω , j , k , p , r .

Recall that the operator $A: Y \rightarrow Y$ is called pseudo-monotone [11, 32] if it is bounded and for any weakly convergent sequence $\{v_k\}_{k=1}^{+\infty}$ in Y to v^* from the inequality $\limsup_{k \rightarrow +\infty} (Av_k, v_k - v^*)_Y \leq 0$ it follows that $\liminf_{k \rightarrow +\infty} (Av_k, v_k - \zeta)_Y \geq (Av^*, v^* - \zeta)_Y$ for all $\zeta \in Y$.

Theorem 3. *The operator $A: W \rightarrow W$ defined by relations (2), (4) is pseudo-monotone.*

We say that the operator $A: Y \rightarrow Y$ satisfies a property of the of bounded Lipschitz continuity type (see [17, 33], compare with [34]) if

$$\|AU - AZ\|_Y \leq \mu(R)\Phi(\|U - Z\|_Y) \quad \forall U, Z \in Y, \quad (7)$$

where $R = \max\{\|U\|_Y, \|Z\|_Y\}$, μ is a non-decreasing on $[0, +\infty)$ function, $[0, +\infty)$ is a continuous, increasing function that satisfies the conditions $\lim_{\xi \rightarrow +\infty} \Phi(\xi) = +\infty$, $\Phi(0) = 0$.

Theorem 4. *The operator $A: W \rightarrow W$ defined by relations (2), (4) satisfies the property of the type of bounded Lipschitz continuity (7) with functions $\mu(\xi) = c^*(1 + \xi + \xi^2)$, $\Phi(\xi) = \xi$, where the positive constant c^* depends on a , G_{13} , E_3 , t , $E^{(k)}$, $v_{12}^{(k)}$, $v_{21}^{(k)}$, $h_{(k)}$, $k = 1, 2$, and constant in the embedding inequalities (6).*

We will say that an operator is quasi-potential [17, 33, 35] if

$$\int_0^1 [(A(t(U + \hat{U})), U + \hat{U})_Y - (A(t\hat{U}), \hat{U})_Y] dt = \int_0^1 (A(\hat{U} + tU), U)_Y dt \quad \forall U, \hat{U} \in Y.$$

Theorem 5. *The operator $A: W \rightarrow W$ defined by relations (2), (4) is quasipotential.*

Let's introduce a functional $\Psi: W \rightarrow R^1$ by the formula

$$\Psi(U, q^1) = \int_0^1 (A(t(U, q^1)), U, q^1)_W dt \quad \forall (U, q^1) \in W. \quad (8)$$

We will say that the functional $\Psi: Y \rightarrow R^1$ is coercive [11, 34], if $\Psi: Y \rightarrow R^1$ for $\Psi: Y \rightarrow R^1$.

Theorem 6. *The functional defined by (8) is coercive.*

From Theorems 3–6, using the technique proposed in [17, 36–42], we can verify that the following theorem holds.

Theorem 7. *Problem (5) has at least one solution.*

5. Iterative method and numerical experiments

For an approximate solving of problem (5), by analogy with [43–48], its finite difference approximation is constructed in the form

$$(A_{1h} + A_{2h})(U_h, q_h^1) = (F_h, 0), \quad (9)$$

where, A_{1h} is a linear operator, A_{2h} is a nonlinear operator.

To solve the difference scheme (9), we will use the following two-layer iterative process with lowering the nonlinearity on the lower layer [49–52]

$$A_{1h} \frac{(U^{(n+1)}, q^{1,(n+1)}) - (U^{(n)}, q^{1,(n)})}{\tau} + (A_{1h} + A_{2h})(U^{(n)}, q^{1,(n)}) = (F_h, 0), \quad (10)$$

where $(U^{(0)}, q^{1,(0)})$ is the given initial approximation, $\tau > 0$ is an iterative parameter. The convergence of finite-dimensional approximations and the iterative method has been studied.

The numerical implementation of the iterative method (10) is being developed. A software package has been developed in Matlab. For the model problem, numerical experiments were performed. The iteration parameter was chosen empirically. The calculations were carried out for the following characteristics: $a = 1$ cm, $h_1 = h_2 = 0.005$ cm, $h = 0.05$ cm, $G_{13} = 15$ MPa, $E_3 = 25$ MPa,

$X_{(1)}^3 = 0.0319$ MPa, $X_{(2)}^3 = 0$, $E^{(k)} = 7 \cdot 10^4$ MPa, $\nu_{12}^{(k)} = \nu_{21}^{(k)} = 0.3$, $X_{(k)}^1 = 0$, $M_{(k)}^1 = 0$, $k = 1, 2$. The number of mesh points is $N = 100$. The initial approximation $(U^{(0)}, q^{1(0)})$ was set to zero. Calculations according to (10) were carried out as long as the residual norm remained greater than the specified accuracy $\varepsilon = 5 \cdot 10^{-6}$. The optimal (in terms of the number of iterations) value of the iteration parameter was $\tau = 1$, the number of iterations being equal to 17.

The results of numerical experiments are shown in Fig. 1–3. It should be noted that the formulated for q^1 boundary conditions correspond to the absence of diaphragms at the edges $x = 0$, $x = a$, which leads to the formation of maximum transverse tangential stresses in the aggregate cross sections at a distance of the order of its thickness $2h$, which is observed in Fig. 2. The restriction of the free displacement of the end sections $x = 0$, $x = a$, in the direction of the axis Ox , leads to the formation in the bearing layers of significant membrane forces $T_{(1)}^{11}$, $T_{(2)}^{11}$, from which the force $T_{(1)}^{11}$ in the cross-section $x = a/2$ turns out to be compressing. Because of this, in the vicinity of this cross section, we should expect a loss of stability of the carrier layers in a mixed form (see [53]), the study of which requires the formulation of the corresponding problem.

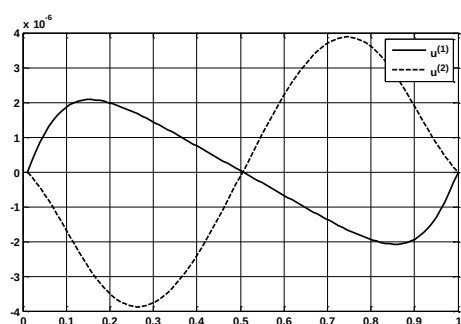


Figure 1. Axial displacements in carrying layers



Figure 2. Tangential stresses in core

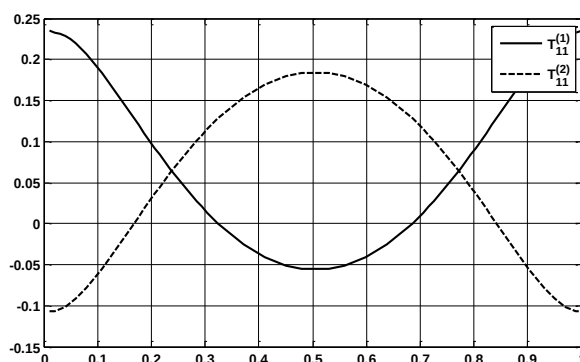


Figure 3. Membrane forces in carrying layers

It is easy to verify that the equality $T_{(1)}^{11} + T_{(2)}^{11} = \text{const}$ holds, which you can be satisfied with on the basis of the results shown in fig. 3.

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